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## **ANALYSIS OF HYDROLOGY OF THE KALIGUNG RIVER AT TEGAL**

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### **ABSTRACT**

Rivers drain water by embracing a gravitational philosophy, in which water always flows from high to low or downstream to downstream. The process of river water flow is an endless natural process, closing the hydrological cycle by restoring the river runoff to the sea. For centuries, rivers have been used as a source of clean water, meeting human needs for drinking water, sanitation, irrigation, and so on. Large rivers are dammed to store water in the wet season and use it in the dry season for various purposes.

Kali Gung or Kaligung or also known as Kaligung river is a river that flows in Tegal regency, Central Java. This river is one of the largest rivers in Tegal besides Kali Ketiwon and Kali Kemiri. This river is called Kali Gung because it is tangent to the spring that comes from Mount Agung is an ancient name from Mount Slamet in pre-Islamic times in Java. Upstream or spring water Kaligung located in the north of Mount Slamet and empties to the north precisely in the sea of Java.

***Keywords:*** *Hydrological cycle, rivers, downstream, upstream, Kaligung.*

**1. BACKGROUND**

Kali Gung or Kaligung or also known as Kaligung river is a river that flows in Tegal regency, Central Java. This river is one of the largest rivers in Tegal besides Kali Ketiwon and Kali Kemiri. This river is called Kali Gung because it is tangent to the spring that comes from Mount Agung is an ancient name from Mount Slamet in pre-Islamic times in Java. Upstream or spring water Kaligung located in the north of Mount Slamet and empties to the north precisely in the sea of Java.

Gung River is located in Gung river basin (DAS) area of 765,625 km<sup>2</sup> which includes Tegal regency and Tegal city with main river length about 55,58 km.

River management, in general, is a technical, administrative, legal, regulatory and management matters concerning river facilities and buildings, Forest observations, maintenance and repair of doors and drainage facilities, flood forecasting, flood warning and countermeasures and others -other. To be able to realize it requires supporting facilities and infrastructure optimally include analyzing River Gung Hydrology.

The study location that will be discussed in this final project report is gung river. Overall DAS gung area of 765,625 km<sup>2</sup> is located in Tegal.

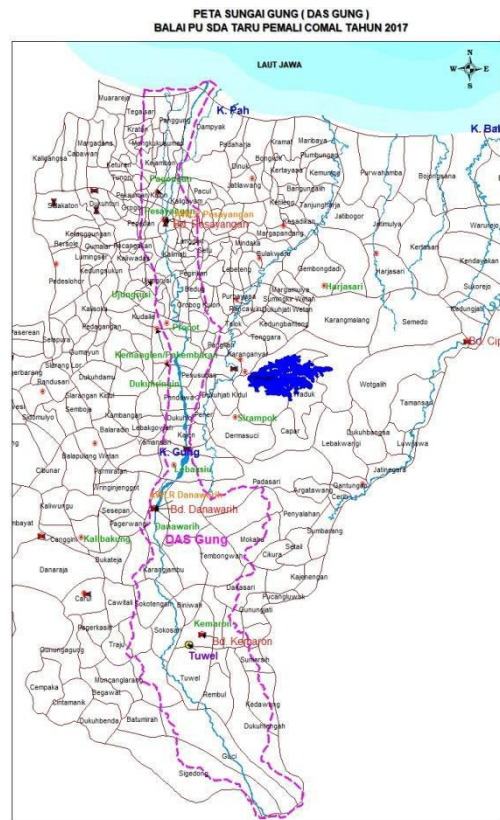


Figure 1. Das Maps of Rivers Gung

**2. FORMULATION OF THE PROBLEM**

The problems and conditions need to be the implementation of the review of countermeasures and improvements as follows:

1. Is the potential for water and discharge sufficient?
2. How much flood discharge the gung river plan?

**3. INTENT AND PURPOSE OF RESEARCH**

The following purposes of the study of River Gung Hydrology Analysis are:

1. Analyzing the discharge to be utilized the potential of water availability optimally, to serve the Irrigation area.
2. Analyzing flood prevention on the river

**4. STUDY AREAS LOCATION**

**5. FRAMEWORK**

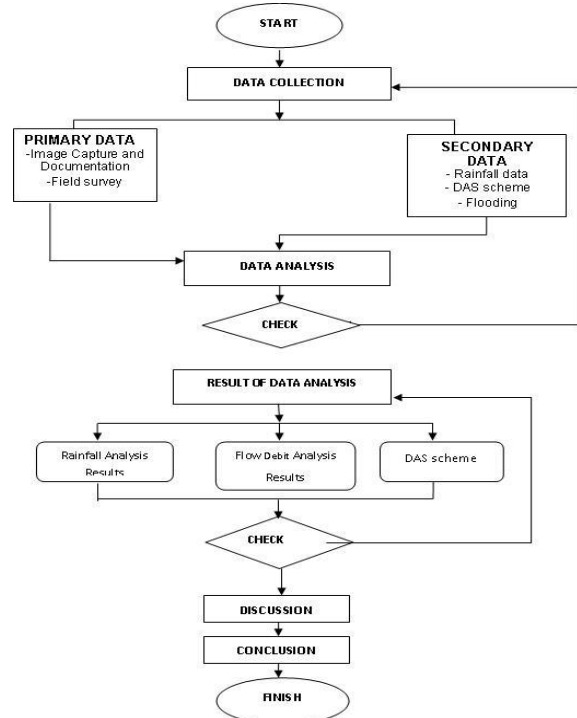


Figure 1. Framework

## **6. THEORETICAL BASIS**

### **8.1 Hydrology**

Hydrology is derived from the Greek Hydrologia which means water science, etymologically hydrology consists of the word hydro and logy where "hydro" means water and "logos" means science, so hydrology can be said water problem. In general, hydrology is the study of the problem of water existence on earth and hydrology itself provides alternative for the development of water resources for the purposes of raw water, industrial agriculture and electricity.

### **8.2 Watershed**

Watersheds are a land area which is a unity with rivers and tributaries, which functions to accommodate, store, and drain water from rainfall to the lake or to the sea naturally, whose boundaries on land are topographical and boundary separators in the sea up to the waters area that is still affected by the mainland activities.

### **8.3 Hydrological Cycle**

The hydrologic cycle is actually very complex and has a wide scope so for the analysis, it is necessary to simplify the model to represent the actual state. To determine the relationship between rainfall, flow and evaporation this can be explained by the hydrological cycle.

### **8.4 River**

The river is a large and long flow of water that flows continuously from upstream (source) to downstream (estuary). Water in rivers is generally collected from precipitation, such as rain, dew, springs, underground runoff, and in certain countries also comes from melt ice/snow. In addition to water, rivers also drain sediment and pollutants.

### **8.5 Rainfall**

Rain is one of the natural phenomena contained in the hydrological cycle and strongly influenced by climate. The existence of rain is very important in life because rain can meet the water needs that are needed by all living things.

### **8.6 Water Availability**

In general in Indonesia, which is a benchmark in irrigation planning is the planning of irrigation water needs for rice crops. The need for rice crops for rice varieties often used in Indonesia is 1 liter/second/hectare average, or the average height of rice puddle is 10 cm.

### **8.7 Flood**

Floods are usually regarded as rising river water levels that exceed their normal state or in the general sense of the overflow of water beyond the normal channel capacity limit. Floods are also defined as large streams of water, flowing water that floods and overflows the normally dry plains.

### **8.8 Plant Patterns**

In cultivating, there are several planting patterns to be efficient and facilitate us in land use and to rearrange the planting calendar. Cropping pattern itself there are three kinds, namely: monoculture, polyculture (tumpangsari), and crop rotation.

## **7. DATA RESEARCH METHODS**

In this study the data obtained by the way, as follows:

1. Looking for data of primary data and secondary data (at the related institution and department) needed to complete data needed for thesis preparation.
2. Study literature as a literature review both from books and other media (internet).
3. Processing and analyzing the data obtained. Taking conclusions and suggestions from the results of thesis studies.

### **7.1 HYDROLOGY ANALYSIS METHOD**

Some of the successful hydrological data collected come from several recording stations, such as manual rainfall scattered in several locations such as Kemaron Station, Lebaksiu Station, Grogol Station, Traju Station, and Pesayangan Station (located in DAS Gung). Hydrological analysis conducted is as follows :

#### **8.5.1 Determination of The Watershed**

Determination of Watershed (DAS) is done based on the 1: 25000 scale DAS map Determination of this area by using Thiessen

polygon method. A number of stations used as many as five rain stations. The need to calculate

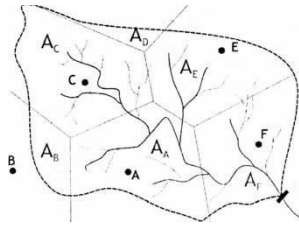


Figure 3. Poligon Thiessen of Watershed

**8.5.2 Mainstay Debit**

For the mainstay discharge Q 80% of the average discharge taken within a certain time (10 years).

$$N = 80 \% \times n$$

Information :

N = year sequence whose Q is used as a mainstay debit

n = number of years of observation

**8.5.3 Availability Debit**

In general in Indonesia, which is a benchmark in irrigation planning is the planning of irrigation water needs for rice crops. Rice water requirement for rice varieties that are often used in Indonesia is an average of 1 liter/second/hectare, or the average height of rice puddle is 10 cm.

**8.5.4 Debit Requirement**

In general, hydrological analysis is carried out:

- water demand analysis for irrigation water demand planning
- calculation of plan debit analysis to determine river cross-sectional capacity in flood control building planning
- hydrological analysis for drainage water planning.
- hydrological analysis for calculation of water potential in order to determine the volume of the reservoir plan
- analysis of the relationship between rainfall and flow rate in order to develop a flood early warning system (Anwar, 2011).

**8.5.5 Flood Debit Analysis Plan**

**A. Rain Distribution According to Monobe**

Average rain from scratch

the area's rainfall is for the preparation of a potential water plan.

$$Rt = \left( \frac{R_{24}}{t} \right) \left( \frac{t}{T} \right)^{2/3}$$

The amount of rainfall to -T

$$RT = t \cdot Rt \cdot (t - 1) \cdot Rt$$

Where

Rt = mean rain intensity.

R24 = Rainfall in 1 day (mm).

T = concentration time (6 hours).

T = Start time of rain (hour)

**B. Method E.J. Gumbel**

With a repeat period of T = 2 years,

T = 10 years, T = 25 years, T = 50 Years, and T = 100 Years and the equations can be seen below.

By formula:

$$X_T = X_r + \frac{Y_T \cdot Y_n}{S_n} S_d$$

Where :

XT = maximum daily rainfall with T year re- period (mm).

Xr = daily mean annual rainfall  $\sum_{n=1}^n X_r$  (mm)

$$Y_T = \text{reduced variate} : Y_T = \left\{ \ln \cdot \ln \left( \frac{T}{T-1} \right) \right\}$$

Yn = Reduced mean.

Sn = Standard deviation.

N = Length of observation year.

**C. Nakayasu Hydrograph Method Unit (HSS)**

Nakayasu of Japan has made the hydrograph formula of the cytentinc unit from the results of his investigation. In the formula as follows:

$$Qp = \frac{1}{3.6} \cdot A \cdot a \cdot \frac{R_0}{(0.3T_p + T_{0.3})}$$

Where :

$$T_p = t_g + 0.8 \cdot t_r$$

$$T_g = 0.21 L^{0.7} \dots \dots \dots L > 15 \text{ km}$$

$$T_g = 0.4 + 0.058 L \dots \dots \dots L < 15 \text{ km}$$

$$T_{0.3} = \alpha \cdot t_g$$

$$a = 0.47 \cdot (A \cdot L)^{0.25} / t_g$$

**D. Metode Der Weduwen**

The calculation steps are

Calculate the magnitude of

flood discharge by re-use the formula :

$$Q_n = \alpha \beta x q x A$$

- $Q_n$  = Flood Debit Plan (m<sup>3</sup> / sec)
- $\alpha$  = Runoff Coefficient / Run Off
- $\beta$  = Coefficient of Reduction
- A = Wide watershed (km<sup>2</sup>)
- q = Rainfall

## 8. ANALYSIS AND DISCUSSION PROBLEMS

### 8.1 DETERMINATION OF RIVER FLOWS

Based on thienesen polygong method results in the broadest catchment area as follows:

Table 1. Wide of DTA

No.	Name of Station	Wide of DTA (Km <sup>2</sup> )
1.	Kemaron Station	237,5
2.	Traju Station	131,25
3.	Pesayangan Station	28,125
4.	Grogol Station	156,25
5.	Lebaksiu Station	212,5
Total		765,625

### 8.2 RAINFALL ANALYSIS

Table 2. The Data of Rainfall from Kemaron Station (Semimonthly) (2007-2016 mm)

Tahun	Januari		Februari		Maret		April		Mei		Juni		Juli		Agustus		September		Oktober		November		Desember		Tahun
	Jan-1	Jan-2	Feb-1	Feb-2	Mar-1	Mar-2	Apr-1	Apr-2	Mei-1	Mei-2	Jun-1	Jun-2	Jul-1	Jul-2	Ag-1	Ag-2	Sep-1	Sep-2	Ok-1	Ok-2	Nov-1	Nov-2	Des-1	Des-2	
2007	108	178	450	165	159	208	262	257	166	83	88	18	1	20	0	7	3	0	11	23	127	129	152	274	2811
2008	249	90	477	157	339	388	63	69	23	81	101	3	0	1	10	10	0	177	182	405	180	177	25	3056	
2009	104	157	189	145	0	0	0	0	0	0	0	0	0	0	0	0	0	0	89	196	102	342	343		
2010	448	500	420	249	289	226	96	241	301	125	195	81	83	124	134	240	267	202	209	369	169	145	451	460	6080
2011	368	532	193	316	185	233	113	292	180	219	0	14	5	23	0	0	0	35	130	429	485	266	251	4279	
2012	344	317	140	551	520	100	380	242	160	100	10	4	0	7	0	0	0	8	156	182	318	317	211	4085	
2013	477	541	273	215	99	207	370	382	171	147	160	83	189	101	0	55	67	27	0	0	90	125	292	180	4150
2014	93	631	437	220	121	248	274	241	141	233	42	186	60	59	129	11	0	0	68	99	163	196	175	3835	
2015	115	174	200	89	278	69	127	144	46	11	33	0	0	0	5	0	0	5	10	106	141	289	195	2083	
2016	89	208	193	316	285	122	112	322	182	185	142	192	59	168	47	16	127	385	70	64	178	120	180	90	3881
Max	477	631	477	551	520	289	382	382	301	233	195	160	189	168	134	240	267	209	369	429	485	451	460		
Rerata	240	335	308	244	226	178	180	216	132	111	69	54	40	50	31	35	58	41	52	100	188	206	242	239	
Min	89	90	140	89	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

Table 3. The Data of Rainfall from Lebaksiu Station (Semimonthly) (2007-2016 mm)

Tahun	Januari		Februari		Maret		April		Mei		Juni		Juli		Agustus		September		Oktober		November		Desember		Tahun
	Jan-1	Jan-2	Feb-1	Feb-2	Mar-1	Mar-2	Apr-1	Apr-2	Mei-1	Mei-2	Jun-1	Jun-2	Jul-1	Jul-2	Ag-1	Ag-2	Sep-1	Sep-2	Ok-1	Ok-2	Nov-1	Nov-2	Des-1	Des-2	
2007	130	164	138	155	115	121	57	147	61	53	64	4	0	34	0	0	0	0	0	5	53	108	70	95	1560
2008	108	238	98	109	134	57	51	84	34	18	17	81	0	0	3	42	8	20	53	133	88	65	84	204	1405
2009	152	232	237	318	122	116	81	94	100	39	0	0	0	0	0	0	0	0	30	0	166	175	124	130	1940
2010	289	249	211	313	233	375	34	166	83	143	145	55	60	79	16	91	20	135	134	142	217	79	108	415	3756
2011	132	187	334	341	280	271	251	40	140	117	18	5	18	4	0	0	0	0	0	171	198	208	85	295	3083
2012	380	288	275	160	675	122	198	85	38	62	40	0	0	0	0	0	0	0	23	85	96	82	330	2753	
2013	232	184	134	95	126	103	163	42	12	17	138	36	123	56	0	22	18	5	0	0	75	89	95	235	1721
2014	129	153	126	343	378	111	229	68	66	101	47	81	120	30	0	0	0	0	0	0	94	113	93	229	2511
2015	382	227	245	110	323	104	40	344	103	36	34	0	0	0	0	0	0	0	0	80	111	244	205	2511	
2016	110	141	334	341	141	24	231	48	55	94	28	99	37	45	14	9	48	194	66	166	84	123	178	1716	
Max	380	288	334	343	675	375	251	244	140	143	145	99	123	75	16	91	48	194	134	171	217	208	244	435	
Rerata	202	200	213	228	253	140	132	113	69	73	53	36	25	3	16	9	25	27	69	119	113	119	250		
Min	108	104	98	95	115	24	34	42	12	18	0	0	0	0	0	0	0	0	0	0	75	65	82	130	

Table 4. The Data of Rainfall from Grogol Station (Semimonthly) (2007-2016 mm)

Tahun	Januari		Februari		Maret		April		Mei		Juni		Juli		Agustus		September		Oktober		November		Desember		Tahun			
	Jan-1	Jan-2	Feb-1	Feb-2	Mar-1	Mar-2	Apr-1	Apr-2	Mei-1	Mei-2	Jun-1	Jun-2	Jul-1	Jul-2	Ag-1	Ag-2	Sep-1	Sep-2	Ok-1	Ok-2	Nov-1	Nov-2	Des-1	Des-2				
2007	104	146	188	147	159	137	76	118	67	65	90	11	7	51	0	0	0	2	39	10	20	164	49	78	112	142	164	1471
2008	113	249	89	102	137	55	34	70	18	2	12	20	0	0	2	39	10	20	164	49	78	112	142	164	1471			
2009	151	279	274	251	107	114	53	76	95	56	13	5	0	0	0	0	0	0	0	74	7	87	176	102	168	1843		
2010	295	214	111	325	91	421	63	182	97	59	382	219	66	45	12	55	28	125	205	99	69	99	169	228	3819			
2011	96	114	312	238	150	216	146	34	90	90	13	7	19	7	0	0	0	0	0	175	220	81	144	345	2425			
2012	241	214	282	99	201	130	88	35	27	38	35	0	0	1	0	0	0	0	20	9	96	153	95	162	2071			
2013	298	295	94	82	149	91	146	53	5	64	164	27	142	59	0	52	30	5	5	84	74	90	179	170	2208			
2014	305	122	141	367	131	84	176	77	66	195	37	70	97	50	2	0	0	0	0	6	81	119	87	207	2166			
2015	287	233	253	254	326	120	52	358	76	54	33	0	0	0	0	0	0	0	0	0	0	0	0	84	142	204	268	2685
2016	92	128	312	238	176	22	146	34	67	116	26	99	35	63	10	10	35	210	88	75	92	152	211	175	2675			
Max	298	279	312	367	326	421	176	358	97	155	382	219	142	63	12	55	35	210	205	175	220	176	311	362				
Rerata	174	195	206	210	163	139	89	104	60	70	81	47	37	28	3	16	10	36	58	55	88	122	155	243				
Min	92	114	89	82	91	22	34	34	5	2	12	0	0	0	0	0	0	0	0	0	69	81	87	164				

Table 5. The Data of Rainfall from Traju Station (Semimonthly) (2007-2016 mm)

Tahun	Januari		Februari		Maret		April		Mei		Juni		Juli		Agustus		September		Oktober		November		Desember		Tahun	
	Jan-1	Jan-2	Feb-1	Feb-2	Mar-1	Mar-2	Apr-1	Apr-2	Mei-1	Mei-2	Jun-1	Jun-2	Jul-1	Jul-2	Ag-1	Ag-2	Sep-1	Sep-2	Ok-1	Ok-2	Nov-1	Nov-2	Des-1	Des-2		
2007	224	166	421	265	115	146	363	235	113	143	188	0	49	37	0	0	0	0	36	44	143	127	97	203	2746	
2008	146	231	259	284	108	56	64	88	6	12	0	5	0	0	0	0	22	7	0	67	15	85	81	156	103	713
2009	133	211	338	177	152	63	99	13	62	3	19	0	0	0	0	0	0	0	23	0	78	72	148	291	1814	
2010	159	355	320	355	188	202	56	265	363	117	254	45	84	96	7	140	207	161	181	156	82	165	223			



8.3.1 Mainstay Debit

Table 6. Potential Calculations from Kemaron Station (wide : 237,5 km<sup>2</sup>) (for example)

Table with 12 columns (Tahun, Jan-1, Jan-2, Feb-1, Feb-2, Mar-1, Mar-2, Apr-1, Apr-2, Mei-1, Mei-2, Jun-1, Jun-2) and 12 rows of data representing monthly calculations for the years 2007-2016.

Table 7. Potential of Calculations Table of Kaligung Watershed

Table with 12 columns (Tahun, Jan-1, Jan-2, Feb-1, Feb-2, Mar-1, Mar-2, Apr-1, Apr-2, Mei-1, Mei-2, Jun-1, Jun-2) and 12 rows of data representing monthly calculations for the years 2007-2016.

Table 8. Potential of Calculation Table of Rice Field (12504 Ha)

Table with 12 columns (Tahun, Jan-1, Jan-2, Feb-1, Feb-2, Mar-1, Mar-2, Apr-1, Apr-2, Mei-1, Mei-2, Jun-1, Jun-2) and 12 rows of data representing monthly calculations for the years 2007-2016.

Table 10. Potential of Calculation Table of Watershed and Irrigation Area

Table with 12 columns (Tahun, Jan-1, Jan-2, Feb-1, Feb-2, Mar-1, Mar-2, Apr-1, Apr-2, Mei-1, Mei-2, Jun-1, Jun-2) and 12 rows of data representing monthly calculations for the years 2007-2016.

Table 11. The Data of Debit Mainstay

Table with 12 columns (No, Januari, Februari, Maret, April, Mei, Juni) and 12 rows of data representing monthly calculations for the years 2007-2016.

8.3.2 Debit Requirement

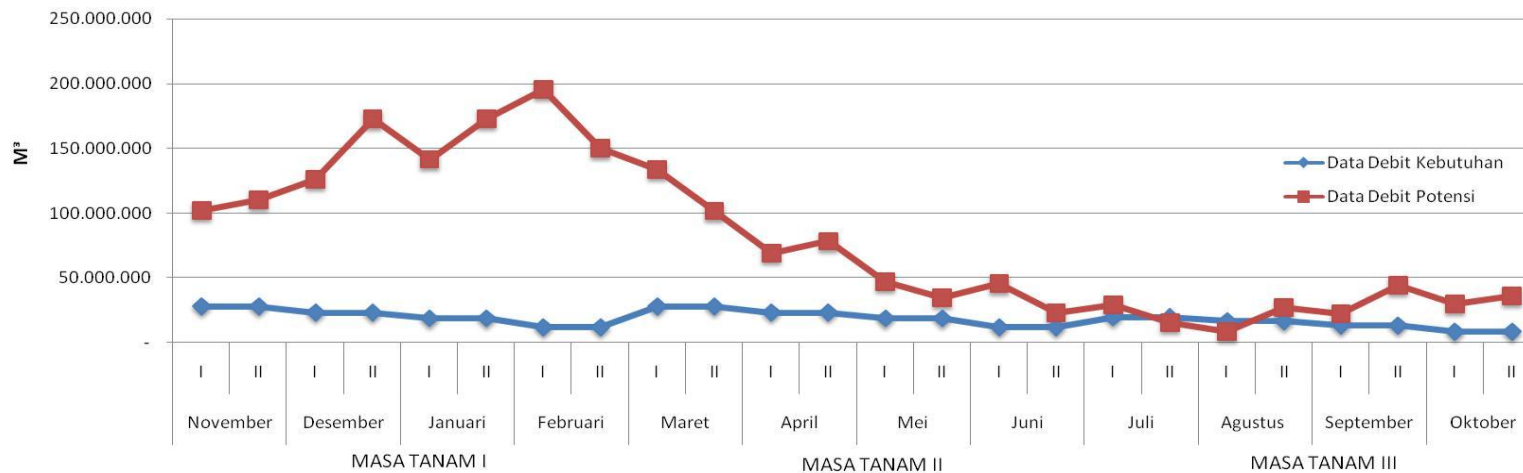
Initially calculated water requirement for 1/2 monthly based on planting area by way of planting area multiplied by coefficient of each subsequent cropping type to obtain water requirement at tertiary door that number multiplied by loss factor in irrigation channel that is 1,25. Then to get the number of water requirement on the secondary door, the above calculation result multiplied by the loss factor in the secondary channel is 1.10. Finally to get the number of water requirement on the primary channel is 1.05.

- Tertiary Water Requirement = CASH x 1.25
• Secondary Water Requirement = CASH x 1.10
• Primary Water Requirement = CASH x 1.05

**Table 9.** Comparison of Debit Requirement and Potential Debit

M <sup>3</sup>	MASA TANAM II							
	Maret		April		Mei		Juni	
	I	II	I	II	I	II	I	II
Data Debit Kebutuhan	28,075,481	28,075,481	23,396,234	23,396,234	18,716,988	18,716,988	11,698,117	11,698,117
Data Debit Potensi	133,706,802	101,645,410	69,145,680	78,338,401	47,029,712	34,642,546	45,484,317	22,997,427
M <sup>3</sup>	MASA TANAM I							
	November		Desember		Januari		Februari	
	I	II	I	II	I	II	I	II
Data Debit Kebutuhan	28,075,481	28,075,481	23,396,234	23,396,234	18,716,988	18,716,988	11,698,117	11,698,117
Data Debit Potensi	102,044,994	110,201,893	126,099,350	172,751,841	141,502,739	172,840,530	195,582,167	149,906,962
M <sup>3</sup>	MASA TANAM III							
	Juli		Agustus		September		Oktober	
	I	II	I	II	I	II	I	II
Data Debit Kebutuhan	19,653,286	19,653,286	16,377,738	16,377,738	13,102,191	13,102,191	8,188,869	8,188,869
Data Debit Potensi	29,037,980	15,515,656	8,400,208	27,045,318	22,304,607	44,514,950	29,843,300	36,068,035

Grafik Perbandingan Debit yang Ada dengan Debit Kebutuhan Setengah Bulanan



**Figure 4.** Comparison Chart Between Available Discharge with Half-monthly Requirement Discharge

**8.4 ANALYSIS OF HYDROLOGY AND RAINFALL**

In Rainfall Analysis which will be used is Gumbel Method, and Weduwen Method.

**8.4.1 Gumbel Method Analysis**

**Table 10.** Resume of Rainfall Rise Calculation Result (R) Gumbel Method

Periode Ulang	Stasiun					Rata-rata
	Kemaron	Lebaksu	Grogol	Traju	Pesayangan	
R2	107	102	116	114	111	107
R5	135	123	154	154	147	135
R10	154	138	179	180	171	154
R25	178	155	210	213	202	178
R50	196	168	234	237	224	196
R100	214	181	257	261	246	214
<b>Rata-rata</b>						<b>164</b>

From the resume, in the average price of max rainfall from the five stations in use Thiesen method to get the rainfall region in each station on each specific repeat period. With an area of  $A = 765,625 \text{ km}^2$ .

**8.5 FLOOD DEBIT ANALYSIS PLAN**

The design debit analysis should use the discharge data from the relevant river but since there is no debit data available, then to analyze the discharge plan will use rainfall data. Flood discharge planning analysis using Weduwen method analysis and HSS Nakayasu method. While the analysis of hourly rainfall using Monobe method.

**8.5.1 Rain-Time Distribution According to Monobe**

**Table 11.** Effective Hourly Rainfall

T (jam)	Distribusi Curah Hujan (%)	Hujan Efektif Dengan Kala Ulang (mm)					
		2	5	10	25	50	100
		107.000	135.000	154.000	178.000	196.000	214.000
1	55.032	58.88	74.29	84.75	97.96	107.86	117.77
2	14.304	15.31	19.31	22.03	25.46	28.04	30.61
3	10.034	10.74	13.55	15.45	17.86	19.67	21.47
4	7.988	8.55	10.78	12.30	14.22	15.66	17.09
5	6.746	7.22	9.11	10.39	12.01	13.22	14.44
6	5.8964	6.31	7.96	9.08	10.50	11.56	12.62

**8.5.2 Metode Der Weduwen**

Be discovered :

I	=	0,002
A	=	765,625
tr	=	27
$\beta$	=	0,81
q	=	2,4
$\alpha$	=	0,54
TR	=	27
R2	=	107
R5	=	135
R10	=	154
R25	=	178
R50	=	196
R100	=	214

Completion :

Q	=	$\frac{\alpha \cdot \beta \cdot q \cdot A \cdot R}{240}$
Q2	=	$\frac{\alpha \cdot \beta \cdot q \cdot A \cdot R}{240} = \frac{0,54 \times 0,81 \times 2,4 \times 765 \times 107}{240} = 358$
Q5	=	$\frac{\alpha \cdot \beta \cdot q \cdot A \cdot R}{240} = \frac{0,54 \times 0,81 \times 2,4 \times 765 \times 135}{240} = 452$
Q10	=	$\frac{\alpha \cdot \beta \cdot q \cdot A \cdot R}{240} = \frac{0,54 \times 0,81 \times 2,4 \times 765 \times 154}{240} = 515$
Q25	=	$\frac{\alpha \cdot \beta \cdot q \cdot A \cdot R}{240} = \frac{0,54 \times 0,81 \times 2,4 \times 765 \times 178}{240} = 596$
Q50	=	$\frac{\alpha \cdot \beta \cdot q \cdot A \cdot R}{240} = \frac{0,54 \times 0,81 \times 2,4 \times 765 \times 196}{240} = 656$
Q100	=	$\frac{\alpha \cdot \beta \cdot q \cdot A \cdot R}{240} = \frac{0,54 \times 0,81 \times 2,4 \times 765 \times 214}{240} = 716$

**Table 12.** Flood Discharge Plan by Weduwen Method

Years	T (hour)	B	q (m3/sec/km2)	$\alpha$	Qn (m3/sec)
2	27	0,81	2,4	0,54	358
5	27	0,81	2,4	0,54	452
10	27	0,81	2,4	0,54	515
25	27	0,81	2,4	0,54	596
50	27	0,81	2,4	0,54	656
100	27	0,81	2,4	0,54	716

**8.5.3 Flood Debit Plan (HSS Nakayasu)**

To get the flood discharge plan, then the up and down hydrograph arch is done in Table 4. The following:

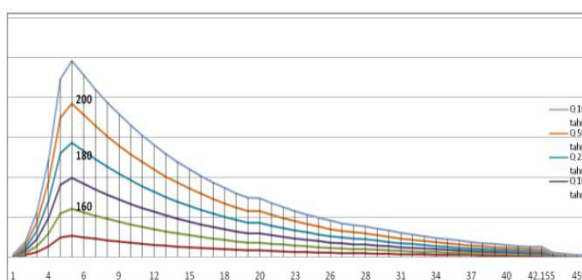
Watershed area (A) = 765 km2 (Result of calculation).

Rever Length (L) 55.58 km ( Results from BBWS map)



**Table 13.** Resume Debit Flood Hydrograf Nakayasu Plan

t	Qt	Q2	Q5	Q10	Q25	Q50	Q100
jam	m <sup>3</sup> /det	m <sup>3</sup> /det	m <sup>3</sup> /det	m <sup>3</sup> /det	m <sup>3</sup> /det	m <sup>3</sup> /det	m <sup>3</sup> /det
0	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
1	0.0203	1.1946	1.5073	1.7194	1.9873	2.1883	2.3893
2	0.1071	7.9442	10.0231	11.4337	13.2156	14.5520	15.8884
3	0.2834	24.0640	30.3611	34.6341	40.0317	44.0798	48.1279
4	0.5652	52.8283	66.6525	76.0332	87.8826	96.7696	105.6566
5	0.9655	97.2196	122.6603	139.9236	161.7298	178.0845	194.4392
5.07	1.0000	107.0001	135.0001	154.0001	178.0002	196.0002	214.0002
6	0.9273	99.2201	125.1843	142.8028	165.0578	181.7490	198.4403
7	0.8550	91.4846	115.4245	131.6694	152.1893	167.5792	182.9692
8	0.7883	84.3521	106.4256	121.4040	140.3241	154.5141	168.7042
9	0.7269	77.7757	98.1282	111.9389	129.3839	142.4677	155.5514
10	0.6702	71.7120	90.4778	103.2117	119.2967	131.3604	143.4241
11	0.6180	66.1211	83.4238	95.1650	109.9959	121.1190	132.2422
12	0.5698	60.9661	76.9198	87.7455	101.4202	111.6762	121.9321
13	0.5254	56.2129	70.9229	80.9046	93.5131	102.9695	112.4259
14	0.4844	51.8304	65.3934	74.5970	86.2225	94.9416	103.6607
15	0.4466	47.7895	60.2951	68.7811	79.5003	87.5396	95.5790
16	0.4118	44.0636	55.5943	63.4187	73.3021	80.7147	88.1273
17	0.3797	40.6283	51.2600	58.4743	67.5872	74.4219	81.2566
18	0.3501	37.4607	47.2636	53.9155	62.3179	68.6197	74.9215
19	0.3228	34.5402	43.5787	49.7120	57.4594	63.2698	69.0803
19.905	0.2999	32.0938	40.4922	46.1911	53.3897	58.7887	64.1877
20	0.2976	31.8473	40.1812	45.8363	52.9796	58.3371	63.6946
21	0.2744	29.3644	37.0485	42.2627	48.8491	53.7889	58.7287
22	0.2530	27.0750	34.1601	38.9678	45.0407	49.5953	54.1500
23	0.2333	24.9641	31.4968	35.9297	41.5291	45.7287	49.9283
24	0.2151	23.0179	29.0412	33.1285	38.2914	42.1635	46.0357
25	0.1983	21.2233	26.7771	30.5457	35.3060	38.8763	42.4466
26	0.1829	19.5686	24.6894	28.1642	32.5535	35.8454	39.1373
27	0.1686	18.0430	22.7645	25.9684	30.0155	33.0507	36.0860
27.622	0.1603	17.1547	21.6437	24.6899	28.5377	31.4235	34.3093
28	0.1555	16.6363	20.9897	23.9438	27.6754	30.4740	33.2726
29	0.1434	15.3393	19.3533	22.0771	25.5177	28.0981	30.6786
30	0.1322	14.1434	17.8444	20.3559	23.5282	25.9075	28.2868
31	0.1219	13.0407	16.4532	18.7689	21.6939	23.8876	26.0814
32	0.1124	12.0240	15.1705	17.3056	20.0026	22.0253	24.0480
33	0.1036	11.0866	13.9877	15.9564	18.4431	20.3081	22.1731
34	0.0955	10.2222	12.8972	14.7124	17.0052	18.7248	20.4444
35	0.0881	9.4253	11.8917	13.5653	15.6794	17.2650	18.8505
36	0.0812	8.6904	10.9646	12.5077	14.4570	15.9189	17.3809
37	0.0749	8.0129	10.1097	11.5326	13.3299	14.6778	16.0258
38	0.0690	7.3882	9.3215	10.6335	12.2906	13.5335	14.7764
39	0.0637	6.8122	8.5948	9.8044	11.3324	12.4784	13.6243
40	0.0587	6.2811	7.9247	9.0400	10.4489	11.5055	12.5621
41	0.0541	5.7914	7.3069	8.3352	9.6342	10.6085	11.5827
42	0.0499	5.3399	6.7372	7.6854	8.8831	9.7814	10.6797
42.155	0.0493	5.2731	6.6530	7.5893	8.7721	9.6591	10.5462
43	0.0460	2.2140	2.7934	3.1865	3.6831	4.0556	4.4280
44	0.0424	1.3920	1.7563	2.0035	2.3157	2.5499	2.7841
45	0.0391	0.9635	1.0895	1.2428	1.4365	1.5818	1.7270
46	0.0361	0.4979	0.6156	0.7022	0.8117	0.8937	0.9758
47	0.0333	0.2098	0.2647	0.3020	0.3491	0.3844	0.4196



**Figure 5.** Chart of Nakayasu

## 9. CONCLUSIONS AND SUGGESTIONS

### 9.1 CONCLUSIONS

From the analysis of the analysis that has been done can be drawn several conclusions as follows:

1. Kaligung River Basin is 765,625 km<sup>2</sup>.
2. From the above comparison results obtained the highest reliable debit occurred in February 195.582.167 m<sup>3</sup> and the lowest potential discharge in August of 8,400.208 m<sup>3</sup>.

3. The results of detailed rain records in the semi-monthly rainfall of the dry season run between May to October and the rainy season between November and April.
4. Modified cropping pattern for Tegal irrigation area three times
  - For Planting Season I (MT I) the need for water for rice crops with planted area of 12,504 hectares, the mainstay discharge is greater than the demand discharge then irrigation water needs can be fulfilled continuously.
  - For the Planting Season II (MT II) water needs for rice crops with planted area of 12,504 hectares.
  - For the Planting Season III (MT III) the need for water for rice crops with an area of 8.753 hectares of total area of 12,504 hectares due to potential discharge at the end of July until early August is insufficient.
  - The flood discharge calculation with method of the Nakayasu was  $Q_{25} = 187 \text{ m}^3/\text{second}$ .

## 10. SUGGESTIONS

Based on the results of the analysis the author can provide suggestions as follows:

1. The availability of water potential from five rainfall stations should be optimized so that the intensity for maximum cultivation.
2. The result of cropping pattern analysis to be a reference or reference for the service
3. Further research is needed, so that the river hydrological analysis can be utilized more optimally
4. The data obtained should be checked whether it is in accordance with the actual conditions.

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